

1 **Monitoring horizontal movement of top of a spire in**  
2 **Saint Mary Cathedral of Burgos via computer vision**

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15     **Abstract**

16           Long-term monitoring of horizontal movement of the top of vertical elements, such as towers of  
17 historical monuments or chimneys, remains a challenge with current solutions. In such structures,  
18 which are usually of high historical and cultural value, ageing and structural degradation directly  
19 impact their horizontal movement.

20           This study presents a novel solution based on computer vision that allows the monitoring of  
21 horizontal movements with very good accuracy (uncertainty less than  $\pm 0.26$  mm), high measurement  
22 speed (up to 5 Hz), and reduced cost.

23           A real-life validation was carried out on the south spire of the Saint Mary Cathedral in Burgos,  
24 Spain. Its horizontal movements were monitored for more than a year, and some relevant events  
25 were recorded, including high-range thermal variations, strong wind events, and ringing of the bells.

26           The results showed that the proposed solution was particularly robust for long-term monitoring  
27 of slow movements, such as those produced by thermal events or those derived from structural  
28 degradation. The white noise of the sensor was small and, for long-term measurements and small  
29 reading frequencies, it could obtain field accuracies of the order of  $\pm 0.1$  mm at a recording frequency  
30 of 1/10 Hz, which further improved at lower recording frequencies.

31     **1. Introduction**

32           During the past decades, there has been a growing interest in the conservation of historical  
33 heritage, especially of religious structures (e.g., churches, cathedrals, and monasteries) as well as  
34 industrial buildings [1-7]. In general, such structures possess very complex geometries and use varied  
35 structural materials (e.g., stone, brick, and wood). In many cases, their historical and heritage value,  
36 and their social and economic importance are very high. However, the costs of repair or  
37 reconstruction can be very high as well, especially if the work is carried out late, i.e., when the  
38 structural damage is more than evident. In addition, this type of work is often very complex owing  
39 to the need to preserve the splendour of the building. For example, it is often essential to use the same  
40 materials and even the same construction techniques as were used when the structure was built.

41           Monitoring of unique structures and those that are particularly vulnerable is a very attractive  
42 strategy, as it can help determine the state of the structure in real-time and plan more accurate  
43 preventive maintenance actions, which are much more economical. However, monitoring such  
44 structures is a difficult task. Firstly, the design of a sensor network is complex owing to their  
45 structural complexity. Numerous types of sensors (accelerometers, inclinometers, strain gauges,  
46 thermocouples, etc.) can be arranged in different locations. In addition, the complexity of data  
47 analysis and post-processing grows exponentially with the number of sensors, which renders  
48 decision-making more difficult. This often leads to shelving of the monitoring of these unique  
49 constructions [8-13].

50           In the case of historic buildings with tall vertical elements, such as towers or spires in churches,  
51 cathedrals, minarets, and chimneys, monitoring could be significantly simplified because, in general,  
52 most of the pathologies in these buildings that could lead to their collapse cause horizontal movement  
53 of the top of the vertical element. Thus, for example, a differential settlement in the foundation of the  
54 building leads to a rotation of the tower and, consequently, a horizontal movement at the top of the  
55 building. In addition, if one of the load-bearing walls of the building partially sinks, it would lead to  
56 a rotation of the tower and a consequent horizontal movement of the top of the tower [14-20].

57           The horizontal movement of the top of a tower or spire in a historic building can be a good  
58 indicator of the structural health of the entire building, especially when its value is found to be  
59 outside the range typically produced by environmental conditions.

60           However, a technical challenge is how to monitor this horizontal movement accurately, for  
61 which the measurement system should possess the following essential characteristics:

- 62           1. Accuracy of the system to be able to provide sub-mm accuracy.
- 63           2. Robustness to withstand varying temperatures and humidity, and operate in harsh  
64 conditions.
- 65           3. Low invasiveness, especially for monitoring buildings of high historical value.

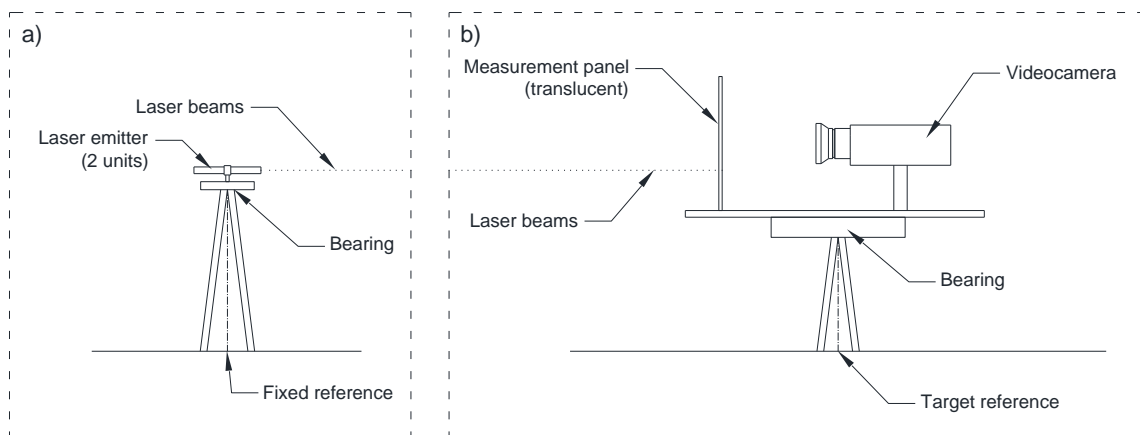
- 66 4. Possibility of autonomous data collection to reduce on-site labour costs.  
 67 5. Reliability to yield accurate and repeatable measurements.  
 68 6. Low cost owing to the large number of historic buildings to be monitored.

69 There are currently some technical solutions on the market, such as laser distance meters, laser  
 70 interferometers, total stations, global positioning system (GPS)-based systems, geophones, and  
 71 accelerometers. However, none of them can meet all the desired requirements.

72 Up to date, most of the research work related to the monitoring of these vertical elements focuses  
 73 on the study of their dynamic response, instead of their static response. However, the information it  
 74 provides used to be insufficient and it is often not useful enough for making decisions related, for  
 75 example, to the need or not to carry out an urgent structural repair [21-25].

76 In recent years, a new technology has emerged, based on computer vision. The development of  
 77 this technology has been progressing very fast, driven by a continuous improvement of the  
 78 characteristics of digital video cameras, which has resulted in rapidly decreasing costs.  
 79 Simultaneously, the increase in the computing power of computers makes it possible to analyse  
 80 images in real-time [26-33].

81 Within this field, the work developed by Vicente et al. [34, 35] is noteworthy. They developed  
 82 a system, namely a laser and video-based displacement transducer (LVBDT), which is based on the  
 83 combined use of a laser (typically one or two laser beams), which acts as a fixed reference, and a video  
 84 camera to monitor the movements (Figure 1).



85 **Figure 1.** LVBDT: a) fixed part and b) movable part [35].

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 87  
 88 This system has been tested to date on a few bridges and a building; however, measurements of  
 89 short duration (lasting only a few minutes) only have been carried out. The above-mentioned studies  
 90 have established the feasibility of this technology. However, to date, it has not been tested in a real  
 91 environment over a long time; furthermore, It has never been used to monitor movements in a  
 92 historic building.

93 There are a few examples in relevant literature on drift monitoring of vertical elements (such as  
 94 towers, spires, and minarets). In this context, some techniques are available that allow the detection  
 95 of these movements. One of them is terrestrial laser scanning (TSL) [36]. This solution provides  
 96 interesting data; however, it is less accurate and more expensive than the one presented in this work.  
 97 Another solution is photogrammetry [37, 38]; even though it is a low-cost solution, it suffers from an  
 98 important uncertainty in that its sampling frequency is very low.

99 This study demonstrates the monitoring of the south spire of the Cathedral of Saint Mary of  
 100 Burgos, Spain, carried out intermittently for more than a year, to test the LVBDT sensor, described in  
 101 detail in [34, 35], in a real environment over a long period of time.

102 The remainder of this article is structured as follows. Section 2 describes the monitored structure  
 103 and the monitoring technology solution adopted, which included an LVBDT sensor and other  
 104 conventional sensors. Section 3 presents the results obtained from the data recorded during certain  
 105 environmental events. Finally, a summary and conclusions are presented in Section 4.

106 **2. Description of the sensor**

107 The sensor used in this project is very similar to the one described in [34, 35], with some  
108 differences as described below.

109 Firstly, the fixed part of the system consisted of a single laser emitter because the objective was  
110 to monitor only the displacements; therefore, a single device was sufficient. The emitter used was a  
111 laser module (FP-LR-250-25-C-F, Laser Components GmbH, Olching, Germany). It was a green diode  
112 laser with a wavelength of 520 nm and an output power of 25 mW. The laser was powered by a 220-  
113 V alternating current. Additionally, it was fixed to the needle wall by means of a precision holder  
114 (FP-MP-30, Laser Components GmbH, Olching, Germany) (Figure 2).  
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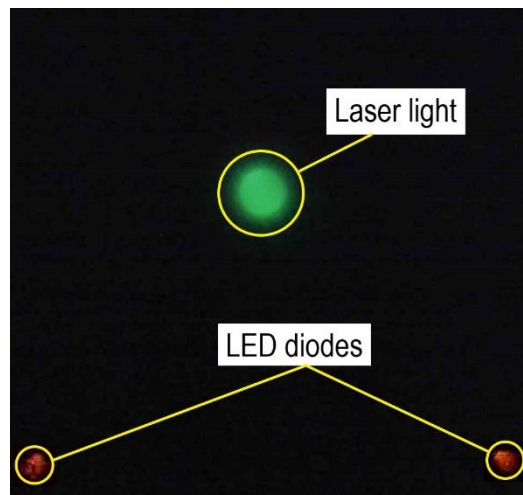
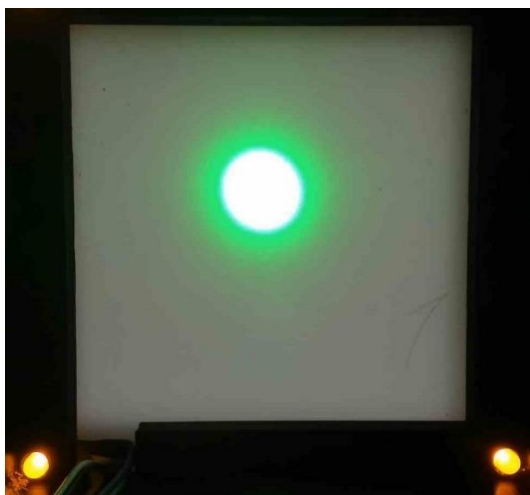


116 **Figure 2.** Laser emitter.

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Secondly, the moving part was made up of several elements. These included a translucent 85 ×  
85 mm panel, on which the laser spot would hit. On the inner side of this panel, there were two red  
LEDs 100 mm apart such that they would define a reference direction to monitor the displacements.  
The LEDs were powered by a 12 V DC current.

Furthermore, both the LEDs and the light spot emitted by the laser were visualised using a video  
camera (Logitech Brio, Lausanne, Switzerland), measuring 27 × 102 × 27 mm and capable of 4K  
resolution (4096 × 2160 px) and a shutter speed of 30 fps. The distance between the webcam lens and  
panel was 130 mm. In addition, a filter was placed over the camera lens to improve detection (Figure  
3).



129 (a) (b)  
130 **Figure 3.** Image filtering using an inactinic welding glass: (a) original image and (b) filtered im-  
131 age.

132 Finally, the camera and translucent panel were housed in a plastic box, creating a compact,  
133 weather-protected unit. This box was designed specifically for this purpose and manufactured from  
134 acrylonitrile butadiene styrene (ABS) using a 3D printing device (Ultimaker S3, Utrecht, Netherlands)  
135 (Figure 4).  
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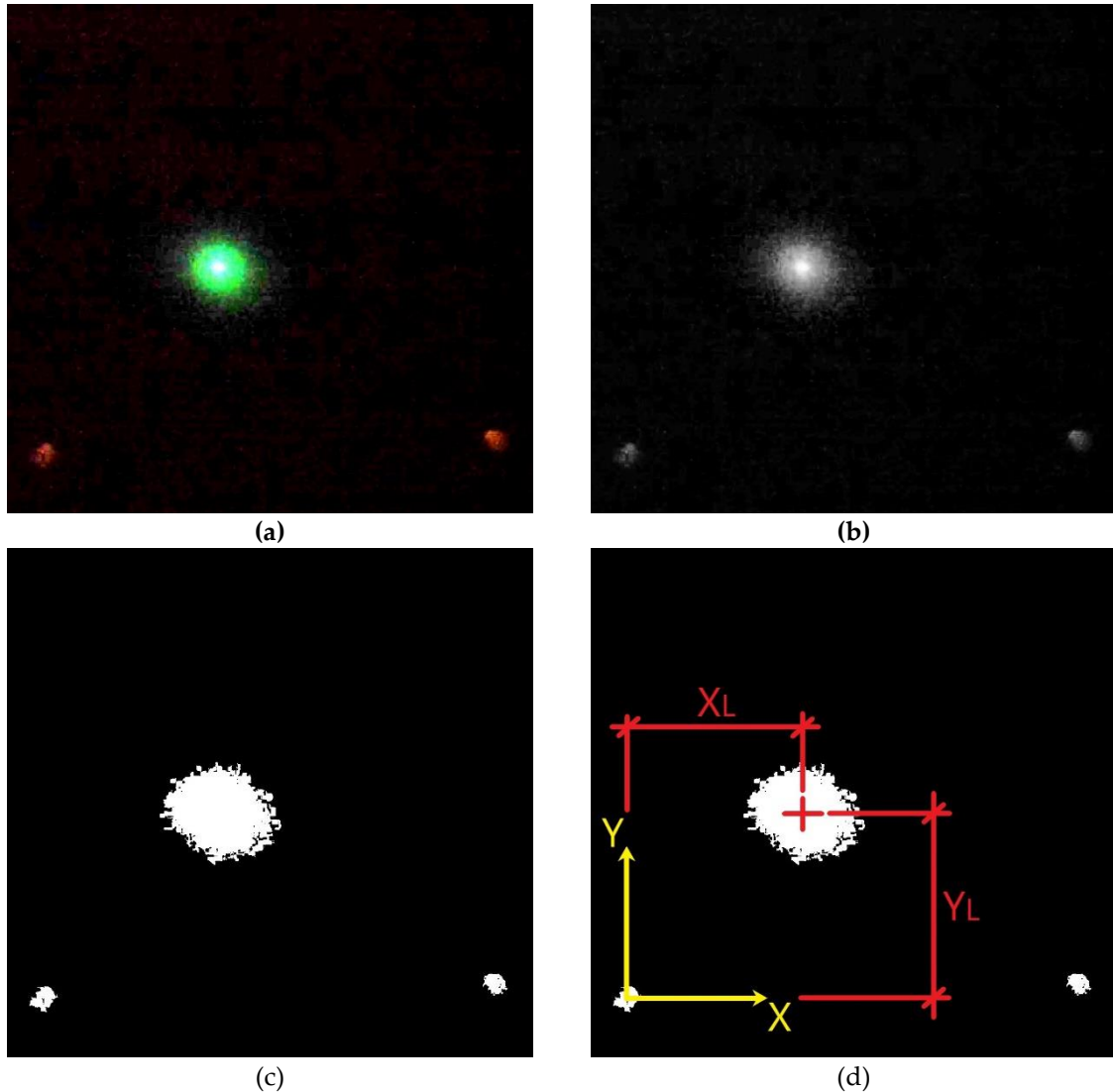
137  
138 **Figure 4.** 3D printed box containing camera and translucent panel.  
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140 The video camera was connected to a computer, and the data were analysed in real-time using  
141 a MATLAB algorithm (MathWorks, Natick, Massachusetts, USA) developed by the authors. The  
142 input data were the frames captured by the camera, which were processed at a rate of 5 Hz. It should  
143 be noted that the camera was previously calibrated to eliminate any distortion. To this end, a  
144 procedure very similar to that described by Brown et al. [34] was followed. Consequently, the frames  
145 processed by the algorithm were corrected using the calibration parameters of the video camera.

146 Figure 5 shows the implemented algorithm, which is further described below. First, the RGB  
147 images of each frame were transformed into grayscale images. Second, to discretise the three light  
148 sources, a threshold grey value was set so that those pixels whose values were lower than this  
149 threshold were considered "background" while those, whose values were higher, were considered  
150 "light sources". This process is called binarisation, as the pixels that make up the image can only have  
151 two values: 0 (black) or 1 (white). In this case, considering that the greyscale images were 8-bit (i.e.,  
152 the greyscale ranges from 0 to 255), the threshold value was set to 10. Therefore, the pixels with a  
153 grey value in the range 0–10 were considered "background", while those with a value in the range  
154 10–255 were considered "light sources".

155 Third, all the pixels that were part of the same light source were merged. The proximity criteria  
156 were used for this purpose. Next, the coordinates of the centroids of each light source were  
157 determined, taking one of the LEDs as the origin of the coordinate system. In this way, the coordinates  
158 of the laser spot, measured in pixels, were obtained directly. The X- and Y- coordinates of the centroid  
159 of each light source were determined as the mean value of the X- and Y- coordinates of the pixels that  
160 belonged to that light source.

161 Finally, to transform the pixels into mm, a scale factor was calculated, considering that the real  
162 distance between the centroids of the two LEDs was 100 mm. This process was repeated for each  
163 frame. In addition, the algorithm contained filtering tools to eliminate digital noise in the images.  
164 More detailed information can be found in Brown et al. [34].



166 **Figure 5.** Algorithm description: (a) original frame, (b) grayscale image, (c) image binarisation,  
 167 and (d) extraction of laser spot centroid coordinates.  
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169 This solution is very robust to changes in light conditions and they do not affect the  
 170 measurement [34]. Convective air currents can slightly modify the laser trajectory and this may  
 171 explain some of the measured white noise. There are also other effects that can also cause white noise,  
 172 such as the internal vibration of the laser emitter.

### 173 3. Experimental campaign

#### 174 3.1. Description of the Cathedral of Burgos

175 During the years 2020 and 2021, the south spire of the Cathedral of Saint Mary of Burgos, Spain  
 176 was monitored intermittently (Figure 6). This is a Catholic church, dedicated to Virgin Mary. Its  
 177 construction began in 1221, following French Gothic patterns. Its construction lasted over 500 years.  
 178 A variety of architectural styles, can be clearly seen in the church, ranging from an early Gothic in the  
 179 first part of the work to the later Gothic in the latest extensions. Today, it is a fine example of Gothic  
 180 architecture in Europe and was declared a UNESCO World Heritage Site in 1984. In 2021, the eighth  
 181 centenary of the beginning of its construction was commemorated.  
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**Figure 6.** General view of the Cathedral of Saint Mary of Burgos, Spain.

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The two spires on the façade of Santa María (Figure 7) stood out for their height. These elements date back to the 15th century and were designed by the German architect Johannes von Köln (Cologne, Germany, 1410 – Burgos, Spain, 1481).



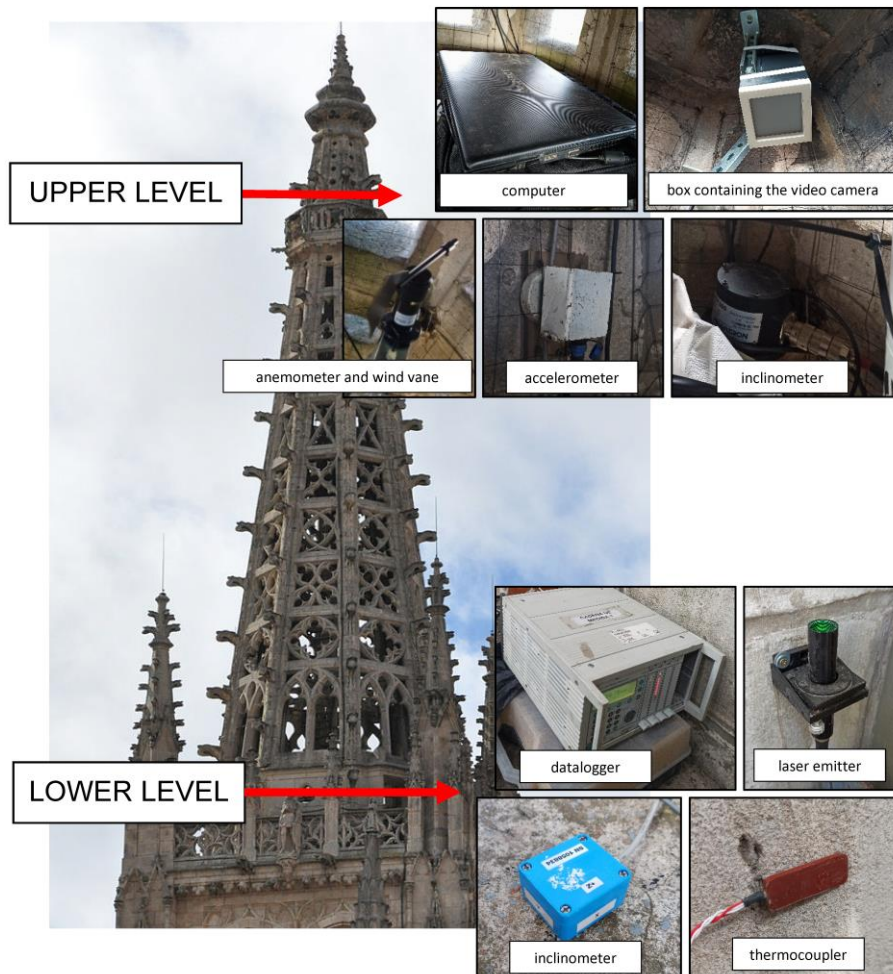
190  
191 **Figure 7.** General view of the south spire placed on the façade of Saint Mary Cathedral.  
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193 The temple is now in an excellent state of conservation, particularly the spires. Therefore, it is a  
194 suitable infrastructure for testing the proposed system for monitoring horizontal movements based  
195 on computer vision.

196 *3.2. Testing equipment*

197 The LVBDT sensor described earlier was used to monitor the horizontal movements of the  
198 southern spire. All the components of the sensor were installed inside the spire so that they were  
199 partially protected from the weather. The laser emitter was located at the base of the spire, where it  
200 is assumed that there is no rotation or displacement, while the translucent panel and video camera  
201 were located at the top of the spire (Figure 8). The distance between the laser emitter and the  
202 translucent panel was 22.2 m.  
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**Figure 8.** General scheme of the sensors and devices placed in the spire.

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Additionally, conventional sensors were used (Figure 8). A biaxial inclinometer model (PST300, PEWATRON AG, Zürich, Switzerland), with a measuring range of  $\pm 5^\circ$  and an absolute accuracy of  $\pm 0.01^\circ$ , was installed in the lower part. Furthermore, a PT100 thermocouple was mounted on the lower part of the spire to monitor the ambient temperature.

At the top of the spire, the following sensors were installed: A biaxial inclinometer (PST300, PEWATRON AG, Zürich, Switzerland); a triaxial accelerometer (4630, TE Connectivity, Ltd., Schaffhausen, Switzerland), with an acceleration range of  $\pm 2g$ , a frequency range of 0–700 Hz, and a sensitivity of 1000 mv/g; and an anemometer coupled with a wind vane to monitor wind speed and direction.

All conventional sensors were connected to a datalogger (MCGPlus, HBM, Darmstadt, Germany) and a laptop computer. In addition, a video camera was directly connected to the computer so that using the algorithm described earlier, the movements of the top of the spire could be evaluated in real-time (Figures 9 and 10).

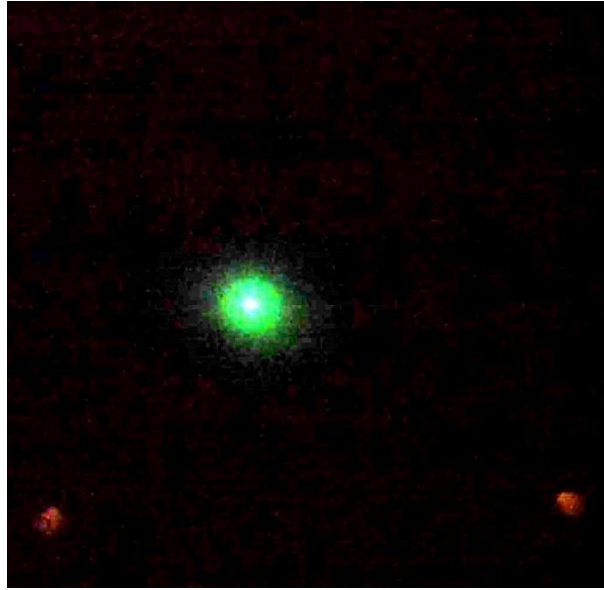


Figure 9. Typical image captured by the camera at a specific instant.

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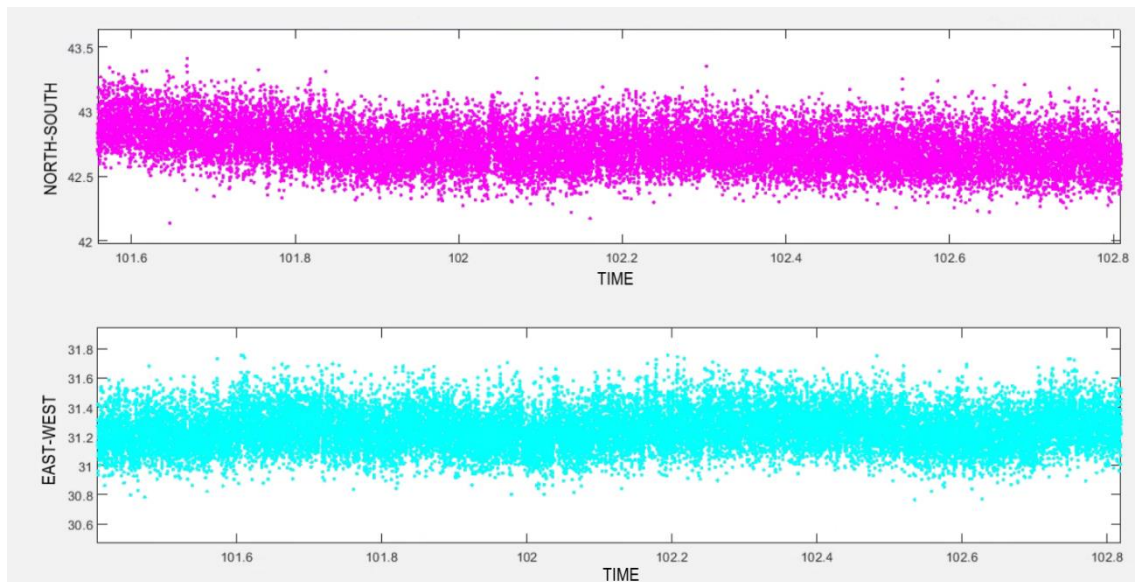


Figure 10. Real-time plotting of graphs with north-south (top) and east-west (bottom) movements. X- axis in seconds, Y- axis in millimetres.

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### 226 3.3. Description of the monitoring process

227 Monitoring was carried out intermittently in 2020 and 2021. Short measurements, lasting a few  
228 days, and longer measurements, lasting several months, were performed. During this time, some  
229 relevant meteorological events were monitored, which are described below.

- 230 1. Thermal variation event. The response of the needle was monitored during 29–31 March,  
231 2021, when the daily thermal variations were particularly relevant.
- 232 2. Wind event. The response of the needle was monitored during the wind storm that took  
233 place during 20-21 March, 2021, with gusts reaching close to 60 km/h.
- 234 3. Induced vibration event. The response of the spire was monitored during the ringing of  
235 bells, which lasted 3 min on 25 May, 2021 at 6:55 PM.

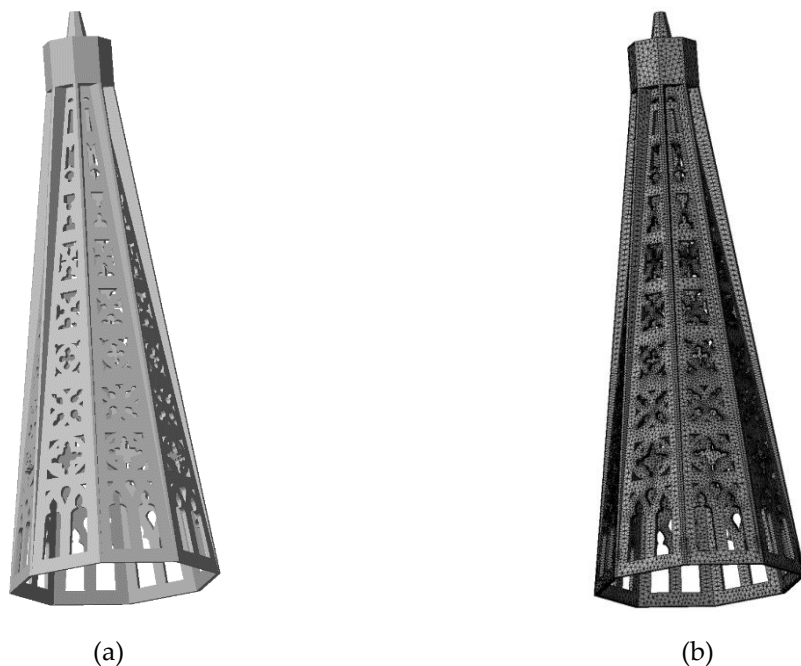
236 Furthermore, to check the robustness of the sensor, a measurement stability test was carried out  
 237 by comparing the measurements on two meteorologically similar days spaced more than one month  
 238 apart. Finally, a test was performed to estimate the white noise of the laser sensor.

239 In each of these events, all the sensors arranged on the spire were recorded. The reading  
 240 frequency of the LVBDT was 5 Hz, and that of the other conventional sensors was 20 Hz.

241 *3.4. Numerical simulation*

242 Additionally, a model of the spire of the cathedral was developed using the finite element  
 243 method (FEM) to estimate the horizontal movements of the top from the data provided by the con-  
 244 ventional sensors, namely the anemometer, wind vane, and thermocouple.

245 The spire was modelled based on the historical construction drawings supplied by the Cathedral  
 246 Chapter. Tetrahedral solid elements of quadratic order were used, with a maximum size of 100 mm.  
 247 In total, the model consisted of  $2.42 \times 10^5$  elements. Figure 11 shows the geometry of the spire and  
 248 mesh.



249 **Figure 11.** Finite element model of the spire: (a) geometry and (b) mesh.

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 251 The boundary conditions imposed on the model consisted of a fixed support (to prevent  
 252 displacements in X-, Y-, and Z- directions) throughout the base, thus simulating the connection of the  
 253 spire with a much more rigid element, such as the rest of the Cathedral tower.

254 The Cathedral was built with limestone from the quarries of Hontoria and Cubillo del Campo,  
 255 located approximately 100 km southwest of Burgos.

256 This limestone was from the Turonian–Campanian period and is not very crystalline; it is dull,  
 257 very uniformly white, pure, massive, and homogeneous; it has a very fine saccharoidal appearance  
 258 and is classified as a very pure packstone limestone containing some fossils (echinids, milliolids, and  
 259 rudists). It is of sedimentary origin and belongs to rudist facies. It is mostly micritic, which allows for  
 260 better weathering.

261 Table 1 lists the physical parameters considered in the numerical models [39-41].

262  
 263 **Table 1.** Main parameters of limestone considered in the numerical models.

Parameter	Value
Density (kg/m <sup>3</sup> )	2200
Thermal conductivity (W/m°C)	2.2

Thermal expansion coefficient ( $^{\circ}\text{C}^{-1}$ )	$1.40 \times 10^{-5}$
Modulus of elasticity (GPa)	30
Poisson's coefficient	0.18

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265 **4. Results**

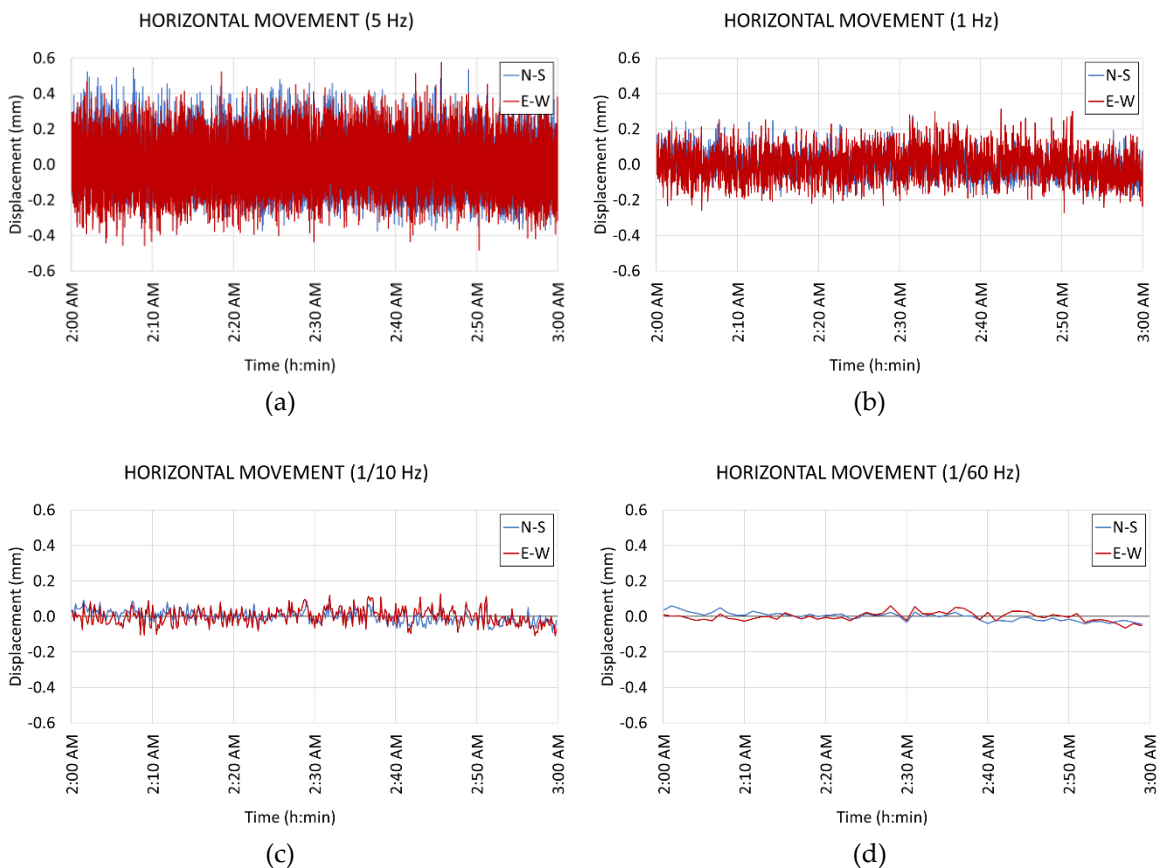
266 The results obtained for each of the events described above as well as their correlation with the  
 267 numerical models employed, are discussed below.

268 *4.1. Estimation of the white noise of the sensor*

269 In the early morning of 23 February, 2021 a one-hour monitoring was carried out (between 2:00  
 270 AM and 3:00 AM), during which the environmental conditions were very stable. The temperature  
 271 was particularly stable in this time slot, with a mean value and maximum variation of 4 and 0.3  $^{\circ}\text{C}$ ,  
 272 respectively. There was a light, steady, south-westerly wind with a wind speed between 4 and 8 km/h.  
 273 Therefore, the environmental conditions were ideal for evaluating the white noise of the LVBBDT  
 274 sensor.

275 Figure 12 shows the horizontal movements at the top of the spire. As indicated above, the  
 276 sampling rate was 5 Hz (Figure 12a). Figure 12b shows the average horizontal movements per second,  
 277 which were obtained by calculating the average value of five recorded data points. Figures 12c and  
 278 12d show the mean values every 10 s and 1 min, respectively.

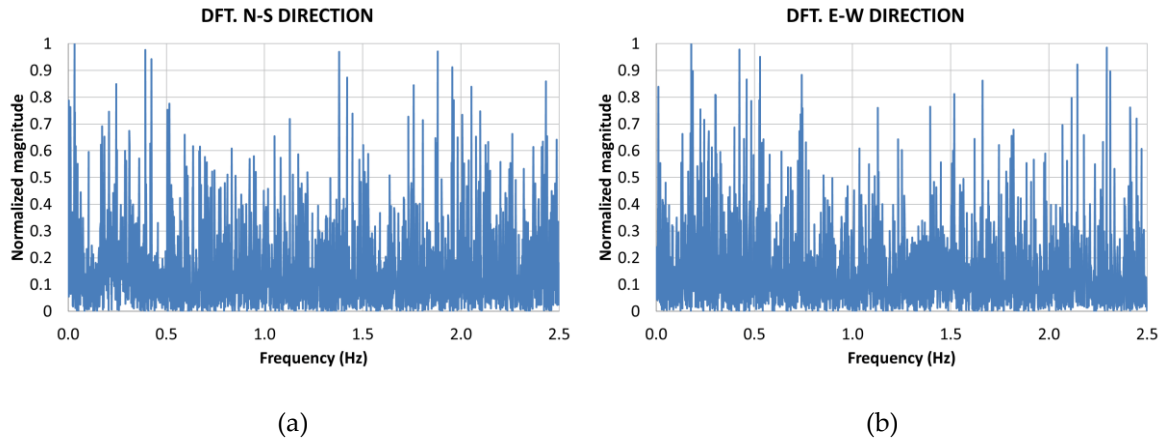
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280 **Figure 12.** White noise at different recording frequencies (Hz): (a) 5, (b) 1, (c) 1/10, and (d) 1/60.

281

282 Additionally, A DFT was performed on the data recorded by the LVBBDT to verify that the signal  
 283 is white noise (Figure 13).



284 **Figure 13.** DFT results from the LVBDT: (a) North-South direction, and (b) East-West direction.

285  
286 Figure 13 demonstrates that the signal is white noise, since there is no dominant frequency  
287 within the analyzed range.

288 White noise, which is stochastic in nature, was considerably reduced when the average values  
289 of longer time periods (lower recording frequencies) were considered. Therefore, as can be seen in  
290 Figure 12, the longer the time period considered, the lower the dispersion of the data. Table 2 shows  
291 the mean values (the measurement was set to zero at the beginning of the test), standard deviation,  
292 and confidence intervals of the measurement for both north–south and east–west orientations.

293  
294 **Table 2.** Statistical parameters of the white-noise event.

Direction	Statistical parameters (in mm)	Recording frequency (Hz)			
		5	1	1/10	1/60
N-S	$\mu$	0.00	0.00	0.00	0.00
	$\sigma$	0.13	0.09	0.05	0.03
	Confidence interval 95%	[0.26,–0.26]	[0.17,–0.17]	[0.09,–0.09]	[0.05,–0.05]
E-W	$\mu$	0.00	0.00	0.00	0.00
	$\sigma$	0.12	0.07	0.04	0.02
	Confidence interval 95%	[0.24,–0.24]	[0.14,–0.14]	[0.07,–0.07]	[0.05,–0.05]

295  
296 As summarised in Table 2, the confidence interval decreased with the recording frequency. For  
297 the case of the highest frequency (5 Hz), the maximum uncertainty occurred in the north–south  
298 direction and was  $\pm 0.26$  mm. In contrast, for the lowest frequency (1/60 Hz or 1 data/min), the  
299 measurement uncertainty was  $\pm 0.05$  mm.

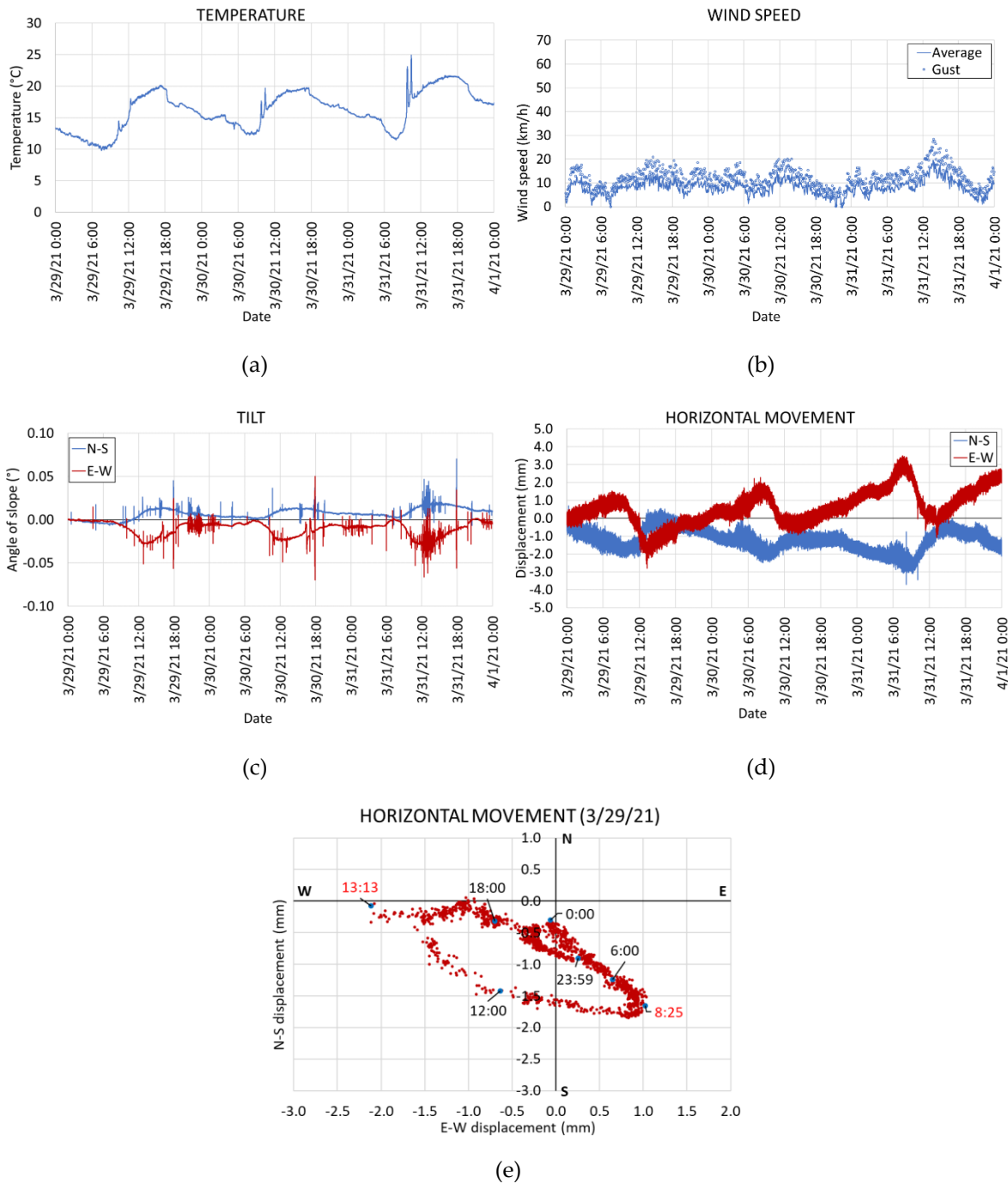
300 In the case of monitoring horizontal movements caused by slow phenomena (such as ageing or  
301 mechanical degradation), the best results would be obtained considering a sampling frequency of 5  
302 Hz and recording the average values at 1/60 Hz. Thus, the highest measurement accuracy could be  
303 achieved.

304 However, in this study, a sampling frequency of 5 Hz was considered for the LVBDT, and then  
305 the average value was considered every second. This option also provided good accuracy.

#### 306 4.2. Thermal variation event

307 A representative thermal variation event took place during 29-31 March, 2021. Figure 13 shows  
308 the measurements recorded by the most significant sensors, namely temperature, wind speed, tilt,  
309 and horizontal movements of the top of the spire.

310



311 **Figure 13.** Measurements during the thermal variation event on 29 March 2021: (a)  
 312 temperature, (b) wind speed, (c) tilt, (d) horizontal movement, and (e) horizontal movement  
 313 according to the cardinal axes.  
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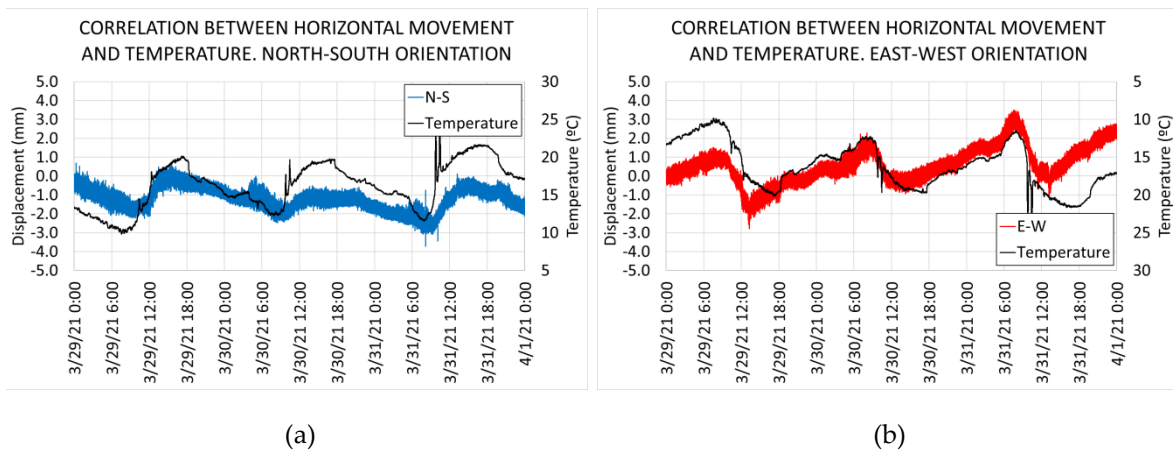
315 As shown in Figure 13a, during this period, the daily thermal oscillation, defined as the  
 316 numerical difference between the maximum and minimum temperature values, was approximately  
 317 10 °C. It is noteworthy that the exceptional temperature peaks occurring every day at approximately  
 318 10:00 AM (especially on 31 March) were not real. This is explained by the fact that during that time,  
 319 the thermocouple received direct sunlight, which abnormally increased the recorded values.  
 320 Therefore, they were not considered in the evaluation of the results.

321 Figure 13b shows the average wind speed during the event, including the gusts. It can be seen  
 322 that the mean wind speed remained approximately constant, with values always below 20 km/h. This  
 323 is important because the wind had an insignificant impact on the results measured by the LVBTD  
 324 sensor.

325 Figure 13c shows the inclinations at the top of the spire. Clearly the tower tilt oscillations were  
 326 of thermal origin. The highest inclinations occurred during the middle hours of the day when the  
 327 solar radiation was the highest. On the other hand, the minimum values were obtained during the  
 328 night, when the temperature was lower. The average maximum inclinations during the event in the  
 329 north–south and east–west directions were 0.03 and 0.05°, respectively. Therefore, it can be concluded  
 330 that the direction of the needle tilt was approximately east–west.

331 On the other hand, Figure 13d shows the displacements at the top of the spire, as measured by  
 332 the LVBDT. Again, it was observed that the thermal variations had a determining influence, as the  
 333 plots were practically homothetic to those in Figure 13a. The maximum variations measured during  
 334 this event in the north–south and east–west directions were approximately 2 and 4 mm, respectively.  
 335 These results agreed with those recorded by the inclinometer, confirming that there was a rotation of  
 336 the spire in the east–west direction. However, there was a fundamental difference between the two  
 337 sensors. While the inclinometer could hardly detect the thermal movements of the structure during  
 338 the night (9 PM to 8 AM), the LVBDT could. This shows that this novel sensor possessed a high  
 339 sensitivity under these circumstances.

340 When the temperature and horizontal displacements were superimposed, a clear correlation  
 341 could be obtained (figure 14).

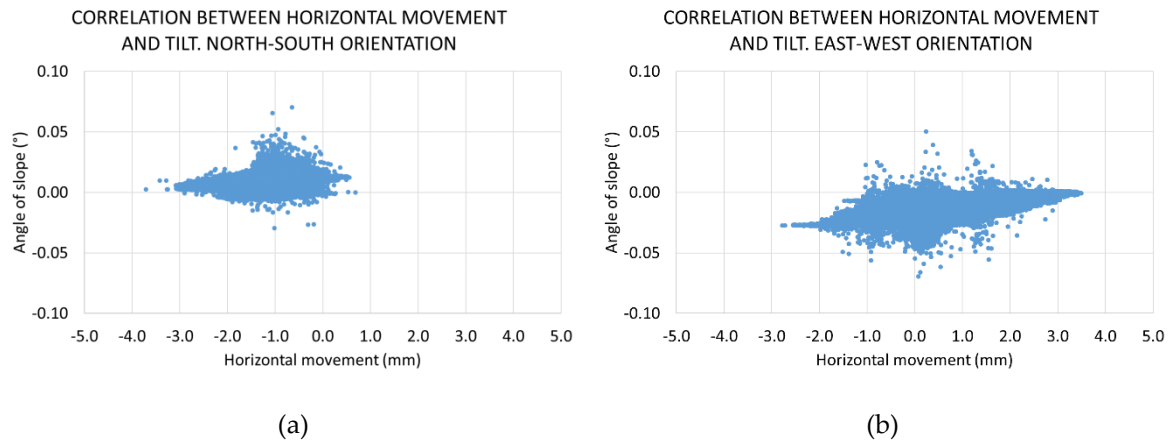


342 **Figure 14.** Correlation between horizontal movement and temperature: a) north–south  
 343 orientation and b) east–west orientation.  
 344

345 In summary, the thermal variations experienced by the spire caused a horizontal movement at  
 346 its top as well as rotation. As observed, the horizontal movements were of the order of a few mm,  
 347 while the rotation was only of a few hundredths of a deg. The LVBDT sensor, which had an accuracy  
 348 of approximately  $\pm 0.2$  mm for a sampling rate of 1 Hz (see Table 2), could accurately measure  
 349 movements of a few mm. However, the inclinometer, which had an accuracy of  $\pm 0.01^\circ$ , did not  
 350 adequately record rotations of a few hundredths of a deg.

351 For a better understanding of the daily rotation of the top of the spire, this movement could be  
 352 represented in terms of the cardinal axes. Figure 13e shows the spatial motion of the needle on 29  
 353 March, 2021. The results clearly reveal that the predominant motion was along the east–west  
 354 direction, again coinciding with the inclinometer data. Furthermore, the westward tilt occurred  
 355 during the period of the highest solar radiation, which in this case, was between 8:25 AM and 1:13  
 356 PM. During this period, the greatest increase in temperature occurred (Figure 13a). On the other  
 357 hand, the eastward tilt slowly recovered during the afternoon and evening, coinciding with the  
 358 decrease in the temperature.

359 Figure 15 shows the correlation between the tilt of the tower and its horizontal movement.  
 360



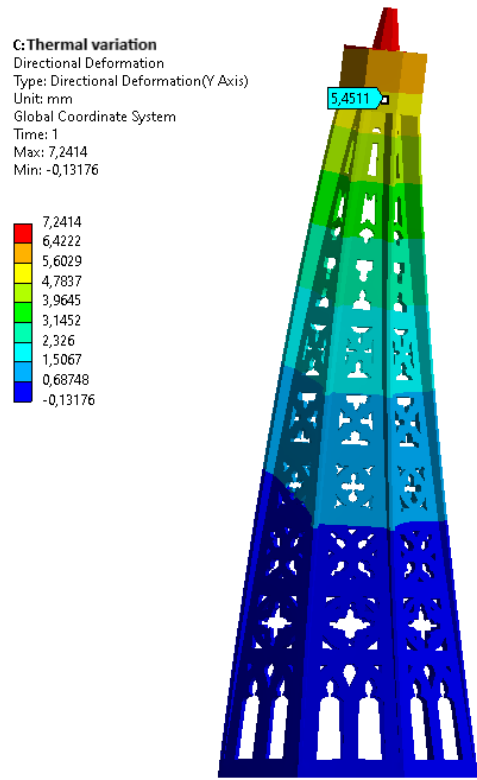
361 **Figure 15.** Correlation between horizontal movement and slope: a) north–south orientation  
 362 and b) east–west orientation.  
 363

364 In both the north–south (Figure 15a) and east–west directions (Figure 15b), a clear relationship  
 365 between both the parameters was observed, which was understandable from a structural point of  
 366 view. In both the cases, the fitting curves were noticeably horizontal, which meant that small turns  
 367 were correlated with large horizontal movements. Given the technical characteristics of both the  
 368 inclinometer and LVBDT, it was found that the latter was more sensitive than the former.

369 In addition, a numerical model was employed to estimate the horizontal movement of the top  
 370 of the spire owing to the measured thermal variation. This model included some simplifications.  
 371 Firstly, a thermal study was carried out to characterise the temperature distribution owing to the  
 372 solar radiation on the surface of the spire. This study was of a stationary nature, that is, assuming  
 373 permanent radiation from the south direction, without considering the rotation of the sun.  
 374 Convective thermal transmittance with the air surrounding the structure of  $10 \text{ W/m}^2\text{K}$  was applied  
 375 at an outside temperature of  $20 \text{ }^\circ\text{C}$ , based on the maximum temperatures recorded by the  
 376 thermocouple (Figure 13a). Consequently, it was determined that the solar radiation produced an  
 377 input of  $300 \text{ W/m}^2$  on the south side, which decreased to  $100 \text{ W/m}^2$  on the east and west sides, and to  
 378 zero on the north side.

379 Second, using the results of the thermal study as input data, a mechanical model was created to  
 380 determine the magnitude of the movements at the head of the spire (Figure 16).





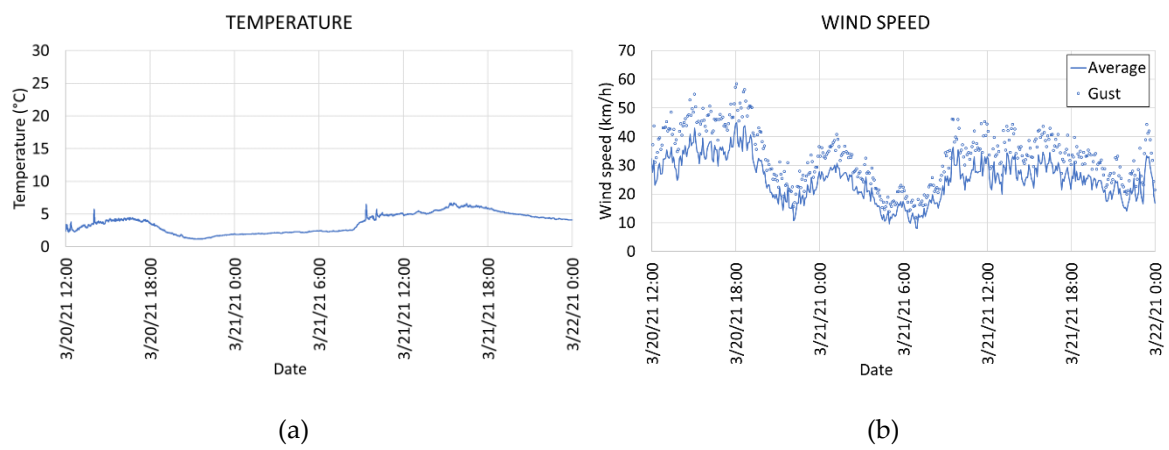
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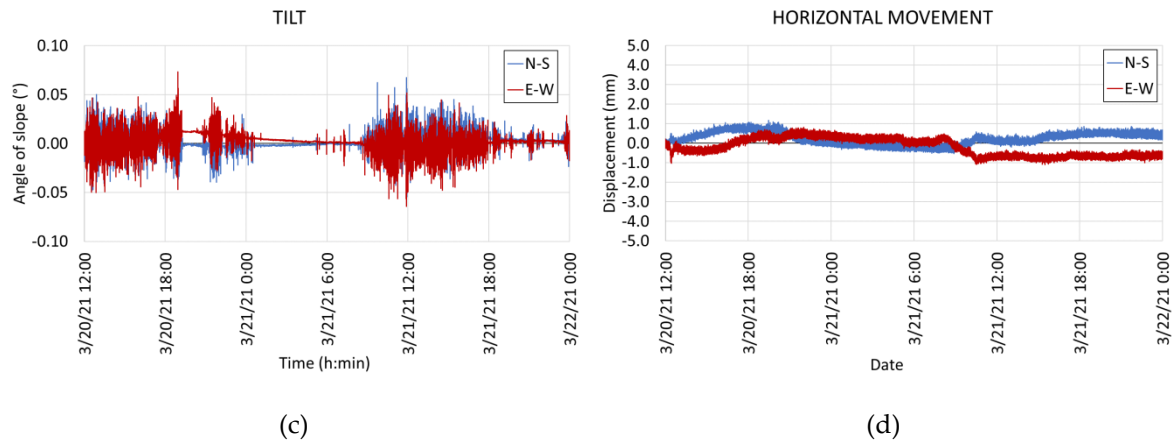
**Figure 16.** Horizontal movement of the spire due to thermal variations as obtained from the FE model.

The numerical model predicted that the maximum horizontal movement at the point where the LVBDT sensor was located was approximately 5.45 mm. This value could be compared with the maximum displacement that occurred at the top of the spire during the thermal variation event; i.e., in Figure 13e, the difference between the two extreme positions (corresponding to 8:25 and 13:13). The result was 3.51 mm; therefore, it could be concluded that the FEM model fit the experimental results considerably, taking into account the assumptions and simplifications made.

**4.3. Wind event**

For the wind event, the response of the spire was monitored during a storm that started in the afternoon of 20 March and ended late on 21 March, 2021. The wind direction was predominantly northeast. The parameters recorded during the wind event are displayed in Figure 17.





396 **Figure 17.** Measurements during the wind event: (a) temperature, (b) wind speed, (c) tilt, and  
 397 (d) horizontal movement.  
 398

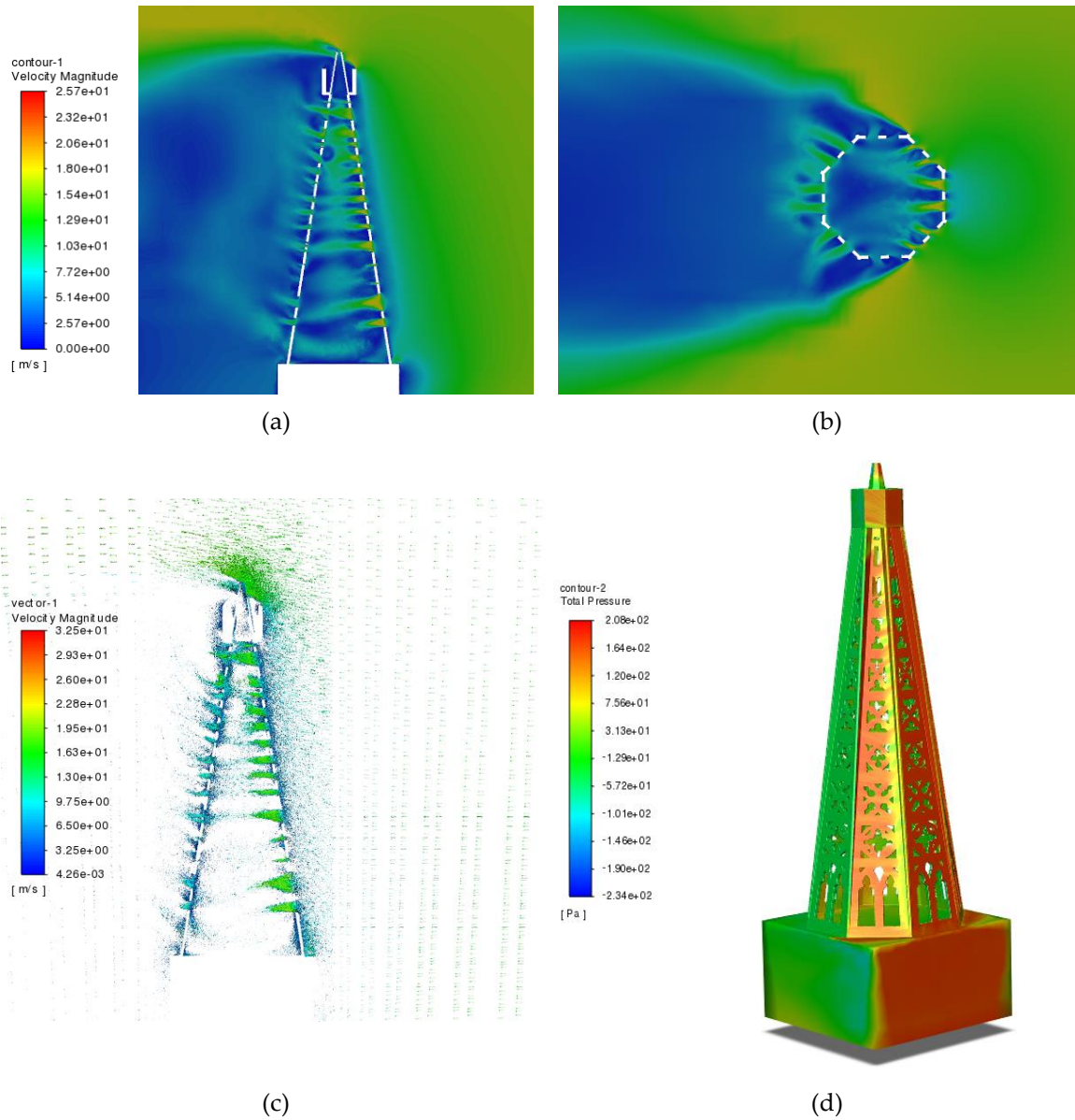
399 As can be observed from Figure 17b, the highest wind gusts occurred during the afternoon of 20  
 400 March, 2021, reaching an average value of approximately 50 km/h. Specifically, an extreme gust  
 401 occurred at 6:04 PM, with a value of 58.4 km/h. 21 March was considerably less windy, with gusts  
 402 of approximately 40 km/h, which further decreased as the day progressed. As for the temperatures  
 403 (Figure 17a), the temperature oscillation on 21 March was 4.7 °C. This was a relatively low value;  
 404 although it would not eliminate the effects of thermal variations, it reduced them, as the focus in this  
 405 case was on the wind.

406 The tilt at the top of the spire is shown in Figure 17c. The graph shows how the largest  
 407 oscillations occurred during periods of highest wind. The movements that occurred were high-  
 408 frequency vibrations of a dynamic nature, without a dominant direction and with an amplitude in  
 409 terms of inclination of approximately  $\pm 0.07^\circ$ . However, there was no correlation between the thermal  
 410 variations and tilt. In this case, the effect of temperature, which caused much slower and continuous  
 411 movements, was overshadowed by the action of the wind.

412 Finally, Figure 17d shows the horizontal displacement of the top of the spire monitored by the  
 413 LVBDT. In this case, no horizontal movement was observed. The maximum horizontal movement  
 414 was approximately 2 mm in both north–south and east–west directions. Furthermore, there was no  
 415 correlation between the wind speed and horizontal movement. In contrast, there was a stronger  
 416 correlation between the temperature variation and horizontal movement. In contrast to the  
 417 inclinometer, the LVBDT recorded the low-frequency movements better; however, it could not record  
 418 the high-frequency movements (i.e., those above 5 Hz in this case, which are typically caused by  
 419 wind).

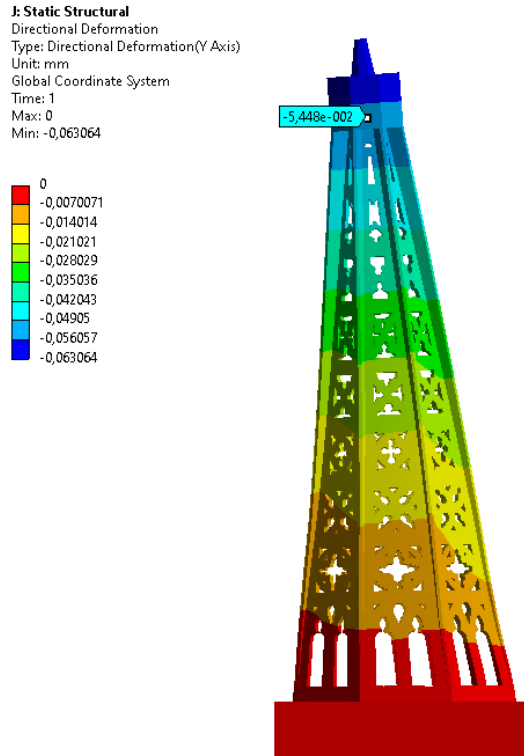
420 The main conclusion from this test was that the LVBDT is especially useful for measuring low-  
 421 or very low-frequency displacements, for example, thermal, rheological, and structural ageing effects.

422 In addition, a numerical model was developed to estimate the theoretical values of the horizontal  
 423 motion of the spire head. Firstly, a computational fluid mechanics (CFD) model was developed,  
 424 considering a uniform wind flow with a velocity of 60 km/h, which approximately coincided with  
 425 the maximum wind values recorded during the wind event (Figure 17b). Figure 18 shows key results  
 426 from the CFD model employed.  
 427



428 **Figure 18.** Key results from the CFD model: (a) wind speed distribution – elevation view, (b)  
 429 wind speed distribution – plan view, (c) wind orientation, and (d) wind pressure on the spire.

430  
 431 From the CFD model results, a mechanical model was developed to determine the horizontal  
 432 movement of the spire (Figure 19).  
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**Figure 19.** Horizontal movement of the spire caused by wind, as obtained from the FE model.

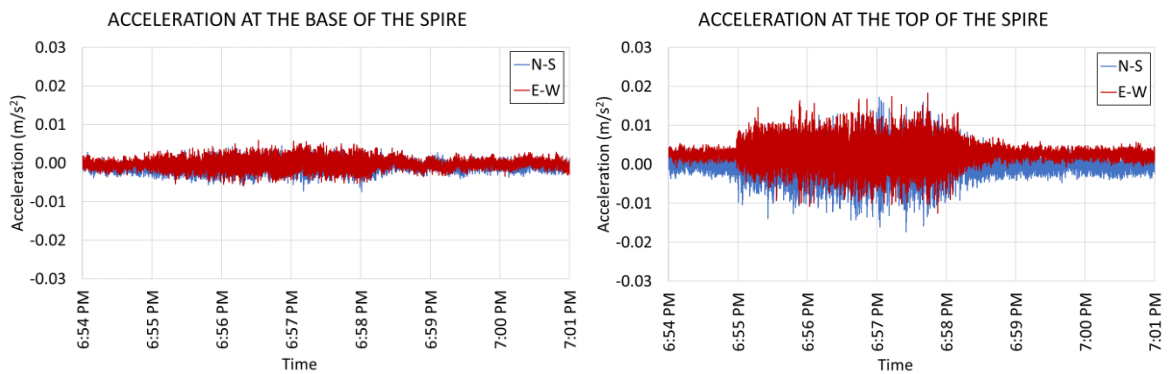
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Figure 19 reveals that the maximum horizontal movement at the point of the spire, where the LVBDT sensor was located was approximately 0.06 mm. This confirms the result, previously deduced from the experimental data, whereby the thermal deformations were larger than those produced by the wind by two orders of magnitude. Consequently, the overall motion of the spire was dominated by thermal actions, whereas low-magnitude, high-frequency wind action was hardly detectable by the LVBDT sensor.

443 *4.4. Induced vibration event*

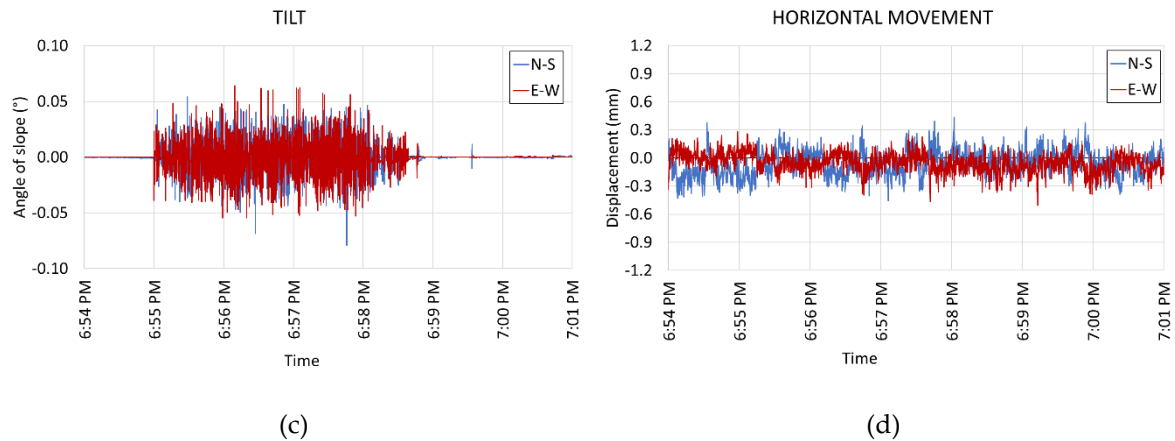
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The induced vibration event took place during the ringing of the bells on 25 May, 2021 which lasted approximately 3 min. The two bells were located under the monitored spire. In this case, the thermocouple and anemometer data were not included, as the duration of the event was too short for the temperature and wind to have a significant effect. It should be noted that the average temperature was 17.5 °C and the average wind speed was 5 km/h, with no notable gusts. On the other hand, the accelerations at the base and top of the spire were included (Figure 20).



(a)

(b)



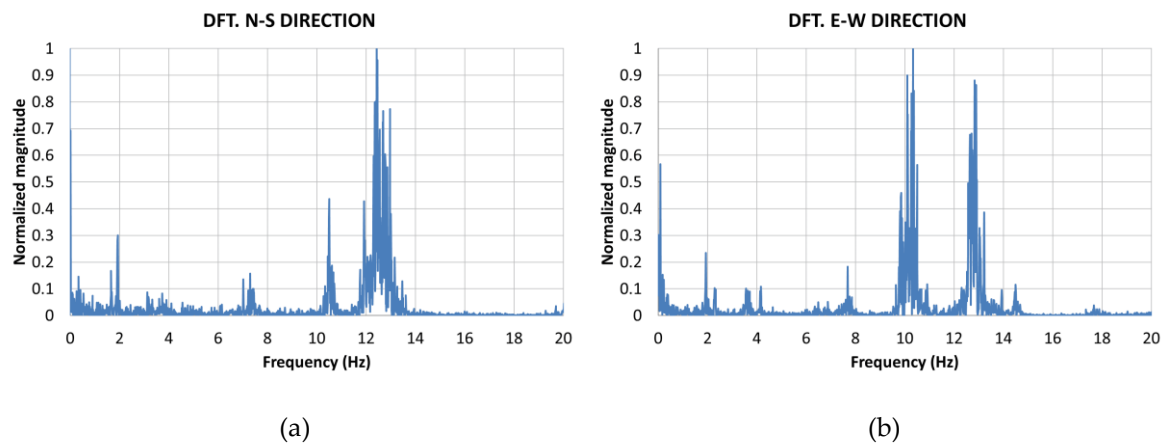
451 **Figure 20.** Measurements during the induced vibration event: (a) acceleration at the base of the  
 452 spire, (b) acceleration at the top of the spire, (c) tilt, and (d) horizontal movement.

453  
 454 The accelerations at the base and top of the spire are shown in Figures 20a and b, respectively. It  
 455 can be observed that the vibrations generated by the bells caused much higher accelerations at the  
 456 apex than at the base. Again, this was to be expected because the behaviour of the spire is equivalent  
 457 to that of a cantilever, and therefore, the greatest movements and accelerations would occur at its free  
 458 end. In this case, since it was a short-term measurement, the sampling rate of the accelerometers was  
 459 50 Hz.

460 Figure 20c shows the tilt at the top of the spire. Similar to the case of wind, the bells caused high-  
 461 frequency vibrations without a predetermined direction. In this event, the amplitude in terms of tilts  
 462 was slightly higher, with an average range of  $\pm 0.06^\circ$ .

463 Finally, Figure 20d shows the horizontal displacement recorded by the LVBDT. Again, the  
 464 results reveal that the sensor could not record the high frequency and very low horizontal  
 465 displacement caused by the ringing of bells. Therefore, it was inferred that the major application of  
 466 the LVBDT was the monitoring of low-frequency displacements.

467 Additionally, A DFT was performed on the data recorded by the accelerometer placed at the top  
 468 of the spire to identify dominant frequencies within the recorded data (Figure 21).



469 **Figure 21.** DFT results from field study: (a) North-South direction, and (b) East-West direction.

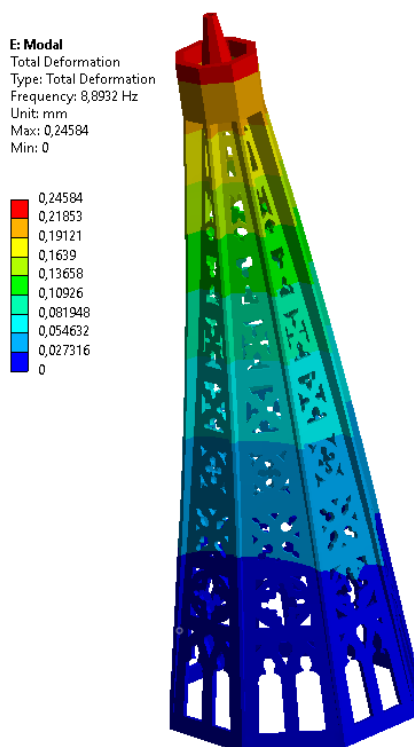
470  
 471 Figure 21a shows that, in the N-S direction, the main natural frequency is 12.43 Hz. Another  
 472 relevant peak frequency can be observed at 10.50 Hz. On the other side, in the E-W direction, the  
 473 main natural frequency is 10.33 Hz, while the other relevant peak frequency can be observed at 12.83  
 474 Hz.

475 In conclusion, the two most important natural frequencies of vibration of the spire are  
 476 approximately 10.50 Hz and 12.50 Hz.

477

478 In addition, a modal study was conducted to determine the natural frequency of the vibration  
 479 of the spire. The first mode of vibration corresponded to a frequency of 8.89 Hz and was associated  
 480 with a cantilever-like behaviour; here, the top of the spire oscillated horizontally (Figure 22).  
 481 However, the frequency of the ringing of bells was approximately 2 – 3 Hz, which is quite different  
 482 from the natural frequency. Therefore, as the experimental data showed, the ringing of the bells had  
 483 a very low dynamic influence on the structure, and the horizontal movements produced were of very  
 484 low magnitude and practically undetectable for the LVBDT.

485 A comparison of the theoretical natural frequency (obtained from the FEM) with the measured  
 486 natural frequencies (see Figure 21) shows that the latter are slightly higher than the former, which  
 487 reveals that the structure is stiffer than that predicted by the model.



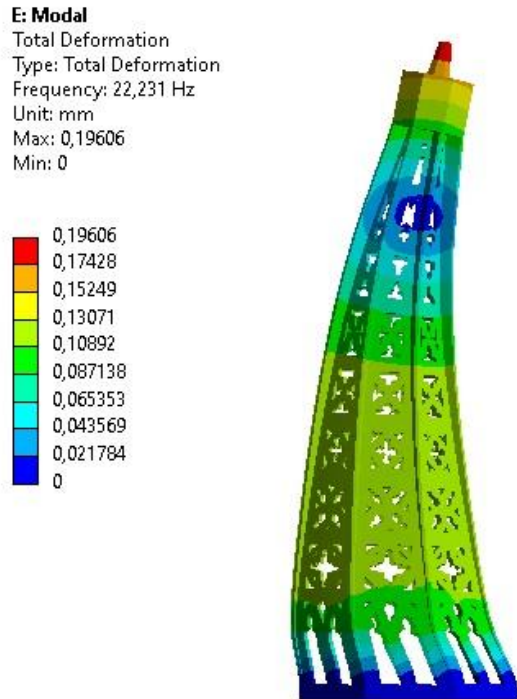
488 **Figure 22.** First mode of vibration of the spire and its corresponding frequency, as obtained  
 489 from the FE model.  
 490

491  
 492 Table 3 shows the main dynamic parameters of the spire.

**Table 3.** Main dynamic parameters of the spire.

Mode	Frequency (Hz)	Period (s)	Participation factor	Ratio	Effective mass	Cumulative mass fraction	Ratio of effective mass to total mass
1	8.89	0.11	6.638	1.000	44.063	0.512	0.459
2	8.89	0.11	3.029	0.456	9.174	0.618	0.096
3	21.14	0.05	0.029	0.004	0.001	0.618	0.000
4	21.27	0.05	0.001	0.000	0.000	0.618	0.000
5	22.23	0.04	5.441	0.820	29.608	0.962	0.308
6	22.24	0.04	1.814	0.273	3.291	1.000	0.034

495  
 496 Table 3 shows that the first mode mobilised a significant part of the total mass (45.9%). The other  
 497 important mode was mode 5, which mobilised an additional 30.8%. Figure 23 shows the fifth mode  
 498 of vibration.



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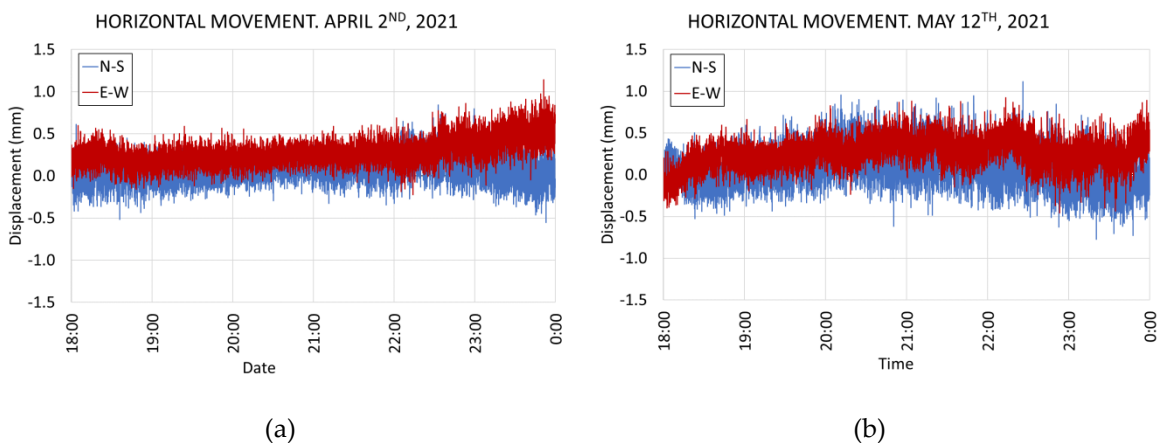
**Figure 23.** Fifth mode of vibration of the spire and its corresponding frequency, as obtained from the FE model.

503 *4.5. Measurement stability test*

504 In addition to the monitoring of meteorological events, a measurement stability test was also  
 505 performed. The objective was to check if there was any drift in the LVBDT measurement; in other  
 506 words, to verify that the measurements remained stable over time. For this purpose, the data  
 507 recorded by the sensor during the afternoons of 02 April and 12 May, 2021 were compared. These are  
 508 two meteorologically similar days, with reduced wind and an average temperature of approximately  
 509 14 °C. Therefore, the spire was assumed to be in the same position on both the days.

510 During these 40 days, the measurement algorithm had not been recording constantly; instead it  
 511 was activated and deactivated numerous times. However, during this entire duration the LVBDT  
 512 was not manipulated.

513 Figure 24 shows the horizontal displacement of the top of the spire, as measured on 02 April  
 514 (Figure 24a) and 12 May (Figure 24b) in 2021, respectively.



515  
 516

**Figure 24.** Horizontal movement measured during two different days: (a) 02 April, 2021 and (b) 12 May, 2021.

517

518 Figure 24 shows that the data were very similar. In both the cases, the mean values were  
519 practically identical and the oscillation was less than 1.5 mm in either direction.

520 It can be concluded that the LVBDT provided a robust long-term measurement, as it was able to  
521 maintain the stability of measurements for 40 days without any appreciable drift. Furthermore, it was  
522 found that as long as neither the fixed nor the mobile part of the sensor was manipulated, it was  
523 possible to de-activate the measurement algorithm or even switch off the computer running it. This  
524 is a very important aspect, as this is often a limitation of other types of sensors.

## 525 5. Summary and conclusions

526 Significant developments in recent years in the field of machine vision have led to the emergence  
527 of a sensor for the measurement of movements based on laser positioning and machine vision,  
528 capable of long-term, highly stable, accurate, robust, and cost-effective monitoring of movements as  
529 complicated as those occurring at the top of a spire of a cathedral.

530 Long-term monitoring of horizontal movements in high-rise structures, such as towers or spires  
531 of churches or cathedrals, minarets, and chimneys is of great interest because most of the structural  
532 degradation phenomena of buildings result in horizontal movement of their tops.

533 The aim of the current study was to evaluate the robustness of the LVBDT laser positioning and  
534 machine vision-based sensor in a real environment, with specific application to the southern spire of  
535 the Cathedral of Saint Mary of Burgos, Spain. This structural element was monitored, on an  
536 intermittent basis, for more than one year, during 2020–2021; different meteorological events were  
537 recorded, and several tests of robustness and accuracy of the measurement system were carried out.

538 In addition to the LVBDT sensor, numerous conventional back-up sensors, including an  
539 inclinometer, accelerometer, a thermocouple, an anemometer and a wind vane were used.  
540 Additionally, two numerical models (a CFD model and a mechanical FE model) were employed to  
541 correlate the measured responses with the expected theoretical values.

542 The results from the sensors showed how the spire, given its characteristics, exhibited significant  
543 horizontal movements of a thermal nature (variations of up to 4 mm for the thermal variation event).  
544 However, the spire hardly suffered any horizontal movement under intense wind events, as it is a  
545 very wind-permeable structure, given its shape and numerous openings; furthermore, it hardly  
546 exhibited any horizontal movement under induced vibrations (in this case, the ringing of the  
547 cathedral bells).

548 The numerical models also yielded interesting results. Firstly, it was observed that the horizontal  
549 movement of the spire was governed by thermal variations, while the action of the wind caused very  
550 small displacements, namely, two orders of magnitude smaller. In addition, the first mode of spire  
551 vibration occurred at a frequency of 8.89 Hz; as a result, the ringing of the bells (with excitation  
552 frequencies below 3 Hz) could not excite the structure; thus, the horizontal movements generated  
553 were very small and undetectable by the LVBDT sensor.

554 The test carried out to measure the white noise of the LVBDT sensor showed that the  
555 measurement uncertainty was very low; moreover, it decreased with the reading frequency. This is  
556 an important aspect, as in the case of long-term monitoring of slowly developing phenomena (such  
557 as those related to structural degradation), the accuracy would be very good (uncertainty of less than  
558  $\pm 0.05$  mm).

559 Finally, the test carried out to evaluate the stability of the measurement showed that the  
560 measurement was stable in the long term, since two measurements taken 40 days apart and under  
561 very similar environmental conditions yielded practically identical values of horizontal movement.  
562 This is an essential requisite for long-term monitoring.

563 It can be concluded that the developed solution yielded excellent field conditions for long-term  
564 monitoring of horizontal movements, especially those of slow development, combining very good  
565 accuracy, high robustness, good long-term stability, and low cost.

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571 logistical support.

## 572 7. Patents

573 A Spanish patent (patent no: ES 2 684 134 B2) was granted [42].

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